



Glulam-Balanced, West Species, 20F-1.5E WS, 8-3/4"x18"
 12 laminations, 8-3/4" maximum width,
 Supports: All - Timber-soft Beam, D.Fir-L No.2
 Total length: 23.05'; Clear span: 22.948'; volume = 25.2 cu.ft.
 Lateral support: top= at supports, bottom= at supports.
 Required fire duration: 1 h; Exposed on two d-faces and one b-face.
 This section PASSES the design code check.

Analysis vs. Allowable Stress and Deflection using NDS 2015:

Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	Fv = 19	Fv' = 195	psi	Fv/Fv' = 0.10
Bending(+)	Fb = 336	Fb' = 1804	psi	Fb/Fb' = 0.19
Live Defl'n	0.15 = < L/999	0.77 = L/360	in	0.19
Total Defl'n	0.22 = < L/999	1.15 = L/240	in	0.19
Fire				
Bending(+)	Fb = 705	Fb' = 5421	psi	Fb/Fb' = 0.13

Additional Data:

FACTORS: F/E (psi) CD CM Ct CL CV Cfu Cr Cfrt Notes Cn*Cvr LC#

Fv'	195	1.00	1.00	1.00	-	-	-	1.00	1.00	1.00	2
Fb'+	2000	1.00	1.00	1.00	0.978	0.902	-	1.00	1.00	1.00	2
Fcp'	425	-	1.00	1.00	-	-	-	1.00	-	-	-
E'	1.5 million	1.00	1.00	-	-	-	-	1.00	-	-	2
Eminy'	0.63 million	1.00	1.00	-	-	-	-	1.00	-	-	2

Only the lesser of CL and CV is applied, as per NDS 5.3.6

Fb'+ (f) 2000 - - - 0.969 0.951 - - - - 4

Only the lesser of CL and CV is applied, as per NDS 5.3.6

CRITICAL LOAD COMBINATIONS:

Shear: LC #2 = D+L, V max = 2300, V design = 1995 lbs

Bending(+): LC #2 = D+L, M = 13225 lbs-ft

Deflection: LC #2 = D+L (live)

LC #2 = D+L (total)

Bending(+): LC #4 = D+L (fire), M = 13225 lbs-ft

D=dead L=live S=snow W=wind I=impact Lr=roof live Lc=concentrated E=earthquake f=fire

All LC's are listed in the Analysis output

Load combinations: ASCE 7-10 / IBC 2015

CALCULATIONS:

Deflection: EI = 6378.65 lb-in²

"Live" deflection is due to all non-dead loads (live, wind, snow...)

Total deflection = 1.5 dead + "live"

Fire: Residual section = 5.15" x 16.20"; Char rate = 1.80 in./hr

Fire design adjustment factor: Fb x 2.85, FbE x 2.03

Lateral stability(+): Lu = 23' Le = 42'-3.81" RB = 10.9

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